LOOK UP AND LIVE

OVERHEAD POWER LINES IN VICINITY OF WORKS.

ENSURE ALL POWER LINES ARE **CLEARLY MARKED WITH ORANGE** WEATHERPROOF TAPE OR RIBBON

ALL WORKS ARE TO BE CARRIED OUT IN ACCORDANCE WITH THE NSW WORK COVER 'WORK NEAR OVERHEAD POWER LINES CODE OF PRACTICE 2006'

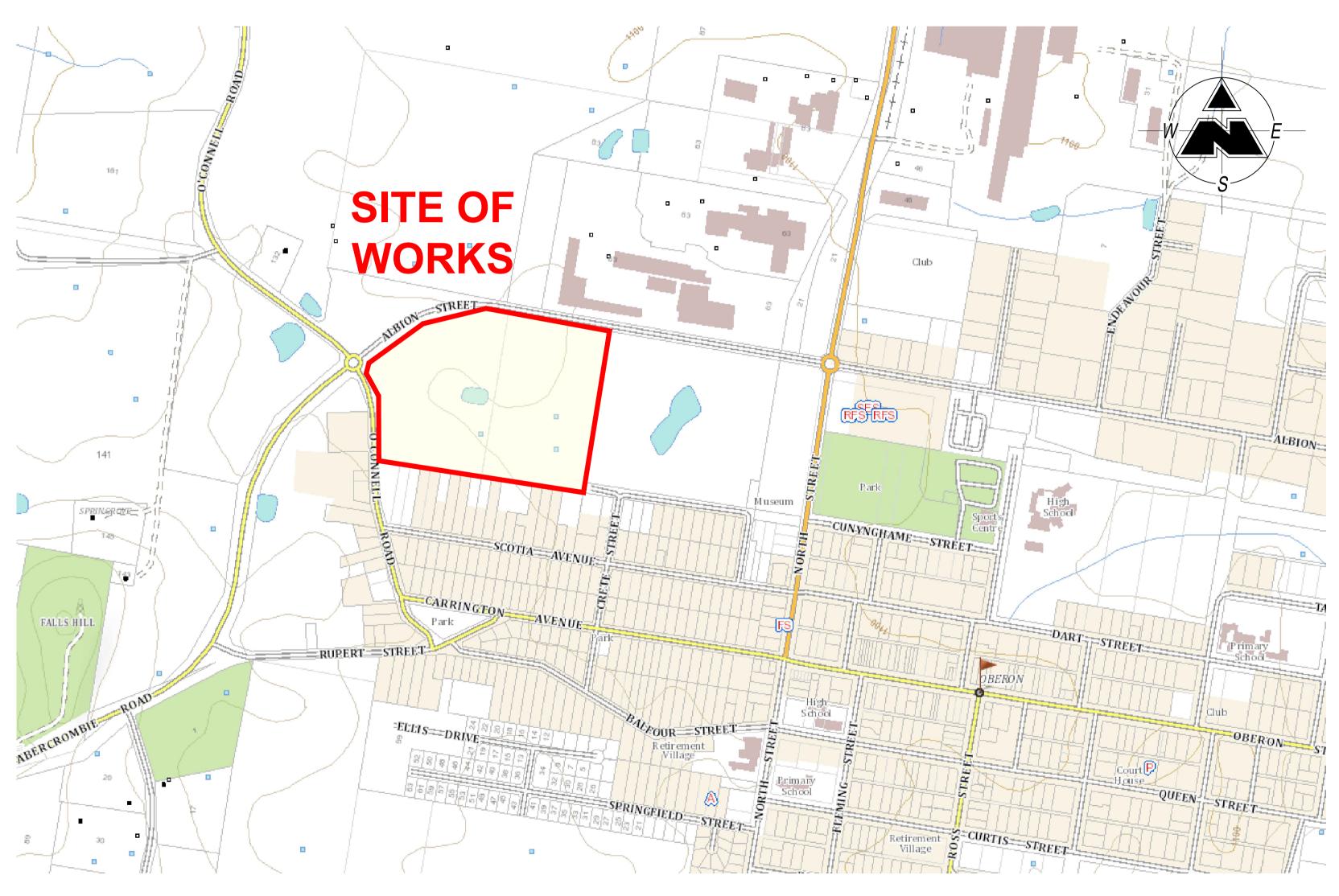
http://www.workcover.nsw.gov.au/health-and -safety/industry-safety/electrical-and-power/ power-lines/publications/work-near-overheadpower-lines-code-of-practice-2006

PROPOSED SPORTS COMPLEX O'CONNELL ROAD **OBERON NSW 2787**

THERE MAY BE EXISTING SERVICES WITHIN THE WORKS AREA THAT ARE NOT SHOWN ON THESE PLANS. THE CONTRACTOR IS TO LOCATE ALL SERVICES PRIOR TO THE COMMENCEMENT OF WORKS.



FOR CROSSMULLER



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LOCALITY PLAN

| PLOT INFO: | \2023.0913-Civil-E.dwg, DATE: Jan 16,2024 - 16:32:12 | |
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| Е | 16/01/24 | AMENDED NOTATIONS | TM | | | | | |
| D | 11/01/24 | FULL PLAN SET - ISSUED FOR APPROVAL | TM | | | | | |
| С | 06/12/23 | BULK EARTHWORKS PLANS - ISSUED FOR APPROVAL | TM | | | | | |
| В | 29/09/23 | REVISED SITE LEVELS AND EARTHWORKS VOLUMES | TM | | | | | Garth Dean |
| A | 25/09/23 | FOR APPROVAL | JB | | | | | B.E. GDSTT FIEAust CPEng NER APEC Engineer IntPE (Aus) RBP |
| Amend | Date | Description | Ву | Amend | Date | Description | Ву | (Vic/NT) |

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25/09/23

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PROPOSED SPORTS COMPLEX O'CONNELL ROAD **OBERON NSW 2787 COVER SHEET**

CROSSMULLER

170 RANKIN STREET,

BATHURST, N.S.W. 2795

Tel: (02) 63323343 Fax: (02) 63318210

Job No. DWG. No. G01

Easement Boundary

Legend - Existing Principal Boundary Abutting Boundary

Contour

Tree - To Be Protected

Road Sign

Stormwater Line Stormwater Swale / Table Drain Stormwater Pit Stormwater Field Inlet Stormwater Manhole Stormwater Headwall

Sewer Line Sewer Maintenance Hole

Water Line Valve Hydrant

Telecommunication Line Fibre Optic Line Telecommunication Pit **Telecommunication Plinth**

Electricity Line Overhead **Electricity Line Underground** Electricity Pole Street Light

Gas Line Gas Valve

Legend - Existing

Bank / Batter **Retaining Wall** Edge Of Seal / Bitumen **Edge Of Gravel**

Legend - Proposed

Edge Of Seal / Bitumen

Bank / Batter

Retaining Wall

Edge Of Gravel

Building Over

Proposed Building

Proposed Concrete

Proposed Gravel

Proposed Field

Proposed Seal/ Bitumen

Proposed Landscaping

Proposed Water/ Basin

Existing Landscaping

Building

—— BH ———— BH ———

Easement Boundary Contour Limit Of Works

Tree

Road Sign

Boundary

Stormwater Line Stormwater Swale / Table Drain Building Hydraulics (Indic. Only) Stormwater House Connection Stormwater Pit Stormwater Field Inlet Stormwater Manhole Stormwater Headwall

Legend - Proposed

Sewer Line Sewer Rising Main **Sewer House Connection** Sewer Maintenance Hole

Water Line Valve **Hydrant**

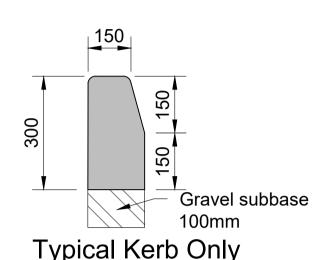
Telecommunication Line Fibre Optic Line Telecommunication Pit **Telecommunication Plinth**

Electricity Line Overhead Electricity Line Underground Electricity Pole Street Light

Gas Line Gas Valve

150 300 1 Layer SL82 mesh 50mm top cover lap mesh 400 elsewhere Waterproof 100mm

Typical Kerb and **Channel Detail** Scale 1:10



(Hobb) Detail

Scale 1:10

NOTE

Based on:

Light Traffic

Total depth

Primer seal

Basecourse

Upper sub-base

Lower sub-base

Wearing course

Bitumen emulsion

The two coat seal wearing surface has been specified at the directive of the developer. Calare Civil recommends that the wearing surface be min. 40mm AC, particularly in the parking areas or other areas of high vehicle manoeuvring. This office takes no responsibility for the performance or longevity of the two coat seal wearing surface.

ACCESS ROAD AND CARPARK

SEALED PAVEMENT CONSTRUCTION DETAILS

As per Table 7.9 of Austroads Pavement Design for

- 325mm

- 2 Coat Seal

- 1.2 l/m² SP30

1m³ to 150m²

- 150mm DGB20

- 175mm DGS40

- Lime Stabilise as directed

DESA of 8x10⁴

CBR 4.5

General Notes

- The builder / contractor is to check and be responsible for the correctness of all dimensions and any discrepancy is to be reported immediately.
- Do not obtain dimensions by scaling off these drawings.
- Stability of the building during construction and excavation in the vicinity of neighbouring buildings is the responsibility of the builder.

All workmanship and materials are to be in accordance with the current S.A.A.

- codes and local government ordinances. Refer architect for set out dimensions, levels, steps and fall
- All standards and codes of practice referred to are those editions current at time of tendering.
- Existing contours are at 0.5m intervals UNO.
- Proposed contours are at 0.25m intervals UNO.
- Existing footpath, kerb and frontage areas to be reinstated to original after completion of works.
- 10. All levels shown are to finished surface level (u.n.o.); edge of concrete lip of kerb invert of drain

Existing Services & Features

- The contractor shall allow for the capping off, excavation and removal (if required) of all existing services in areas affected by works within the contract area or as shown on the drawings unless directed otherwise by the superintendent.
- The contractor shall ensure that at all times services to all buildings not affected by the works are not disrupted.
- Prior to commencement of any works the contractor shall gain approval of his program for the relocation/construction of temporary services.
- Contractor shall construct temporary services to maintain supply to existing building remaining in operation during works to the satisfaction and approval of the superintendent. Once diversion is complete and commissioned, the contractor shall remove all such temporary services and make good to the satisfaction of the superintendent.
- Interruption to supply of existing services shall be done so as not to cause any inconvenience to the principal. Contractor to gain approval from the superintendent for time of interruption.
- Existing services, buildings, external structures and trees shown on these

drawings are existing features prior to any demolition works.

Earthworks

- The contractor is to strip the construction area of all grass, shrubs, rubbish, deleterious material and unsuitable topsoil as nominated by the engineer. Disposal of this unsuitable material is to be off site.
- Topsoil approved by the superintendent for reuse, is to be stockpiled on site
- Bulk earthworks is to be carried out in accordance with council standards and the requirements of AS3798.
- 4. Prior to fill operations and in the presence of the geotechnical engineer, proof roll the fill area subgrade. Remove soft and or compressible zones and replace with select site material compacted to a density consistent with that noted for the proposed filling.
- Proof rolling nominated shall be carried out using a single axle highway truck with a rear axle load not less than 10 tonnes tyres inflated to 550kpa or approved equivalent. Equipment labour and loading required for proof rolling is to be provided by the contractor.
- The majority of material won from proposed excavations should be suitable for use as fill material. Suitable materials for filling should generally have a maximum particle size not exceeding 150mm. Oversize material is to be either crushed to a particle size =/< 150mm for reuse as fill material or be disposed of off site.
- All fill under footings and slabs shall be compacted in layers not greater than 150mm to 100% standard compaction. For cohesive materials or a density index of not less than 70% for non cohesive materials. Tests shall be conducted on fill as required by qualified soil test consultant to confirm compaction. Following completion of bulk earthworks operations the contractor is to notify the superintendent. The finished surface is to be proof rolled in the presence of the geotechnical engineer prior to topsoiling/building.
- 8. It is the contractors responsibility to protect the site and surrounding areas from damage resulting from stormwater runoff. Temporary diversion drains and or other drainage control devices are to be implemented by the contractor during construction to minimise the effects of weather.
- Imported fill material if ordered, shall be low plasticity granular fill having the following characteristics: Minimum CBR 15

Plasticity index <15 %

Passing 0.0075mm sieve <25 %

- 10. Excess spoil material generated during construction is to be disposed of as directed by the superintendent.
- 11. All fill material placed on the site comprising only natural earth and rock is to be free of contaminants (as defined by schedule 1 of the POEO 1997), noxious, hazardous, deleterious and organic materials. Suitable fill material is deemed to comply with the requirements of clause 4.3, AS3798, guidelines on earthworks for commercial and residential developments.
- 12. The movement of material to and from the site is to be in accordance with relevant EPA policies, in particular those addressing presence and treatment of fire ants.

Siteworks Notes

- Datum A.H.D.
- Origin of levels, refer to bench or state survey marks where shown on plan.
- Contractor must verify all dimensions and existing levels on site prior to commencement of work.
- All works to be undertaken in accordance with the details shown on the drawings & the directions of the superintendent.
- Existing services unless shown on survey plan have been plotted from services search plans and as such their accuracy cannot be guaranteed. It is the responsibility of the contractor to establish the location and level of all existing services prior to the commencement of any work. Any discrepancies shall be reported to the superintendent. Clearances shall be obtained from the relevant service authority.
- Where new works abut existing the contractor shall ensure that a smooth even profile, free from abrupt changes is achieved.
- The contractor shall arrange all survey setout to be carried out by a registered
- Care is to be taken when excavating near existing services. No mechanical excavation is to be undertaken over Telstra or electrical services. Hand excavate in these areas.
- Contractor to obtain authority approvals where applicable.
- 10. Make smooth transition to existing surfaces and make good.
- 11. These plans shall be read in conjunction with approved landscape, architectural, structural, hydraulic and mechanical drawings and specifications or written instructions that may be issued relating to development at the site.
- 12. Trenches through existing road and concrete pavements shall be sawcut to

- full depth of concrete and a minimum of 50mm in bituminous paving. 13. All branch gas and water services under driveways and brick paving shall be located in 80Ø uPVC sewer grade conduits extending a minimum of 500mm
- beyond edge of paving. 14. Grades to pavements to be as implied by RL's on plan. Grade evenly between nominated RL's. Areas exhibiting ponding greater than 5mm depth
- will not be accepted/ unless in a designated sag point. 15. All covers and grates etc to existing service utilities are to be adjusted to suit
- new finished surface levels where applicable. 16. All buildings and hardstand areas are to be removed and disposed of off site in accordance with local council requirements.
- 17. All trees and tree waste to be disposed of off site in accordance with local council requirements.

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Approved for Construction: TM 16/01/24 | AMENDED NOTATIONS 11/01/24 | FULL PLAN SET - ISSUED FOR APPROVAL D TM TM BULK EARTHWORKS PLANS - ISSUED FOR APPROVAL Garth Dean TM REVISED SITE LEVELS AND EARTHWORKS VOLUMES 29/09/23 B.E. GDSTT FIEAust CPEng NER 25/09/23 FOR APPROVAL JB APEC Engineer IntPE (Aus) RBP (Vic/NT) Amend Date Description By Amend Description

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PROPOSED SPORTS COMPLEX O'CONNELL ROAD **OBERON NSW 2787**

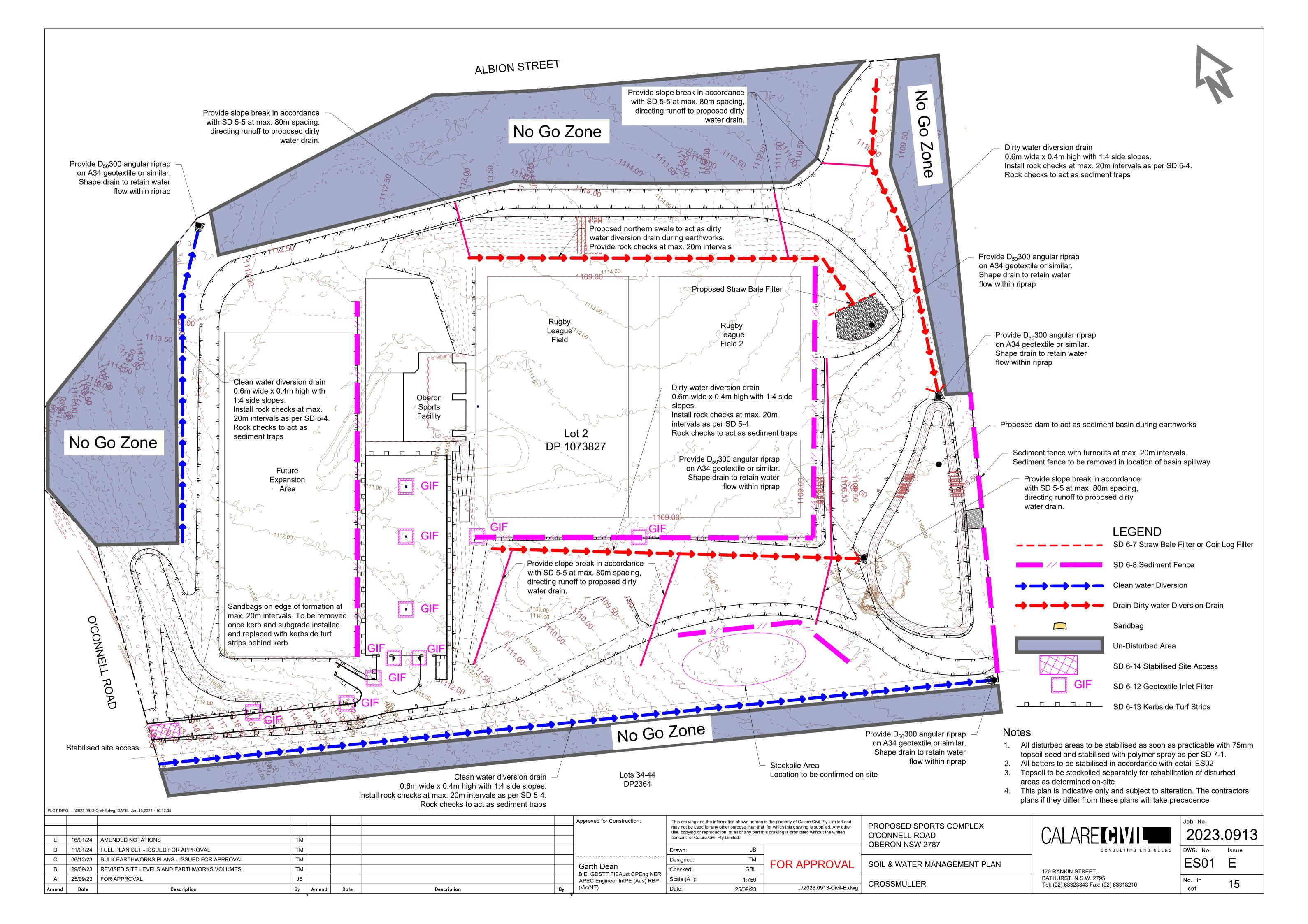
LEGEND AND GENERAL NOTES

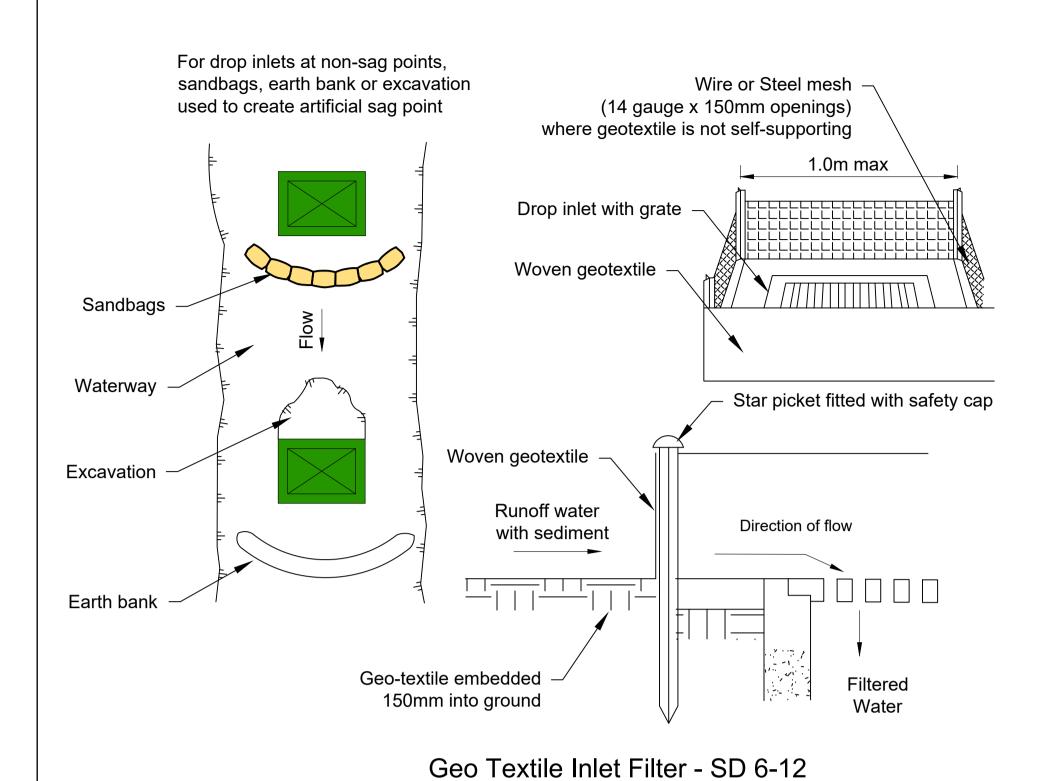
CROSSMULLER

CALARETHIMI

170 RANKIN STREET, BATHURST, N.S.W. 2795 Tel: (02) 63323343 Fax: (02) 63318210 Job No. 2023.0913 DWG. No. G02 15





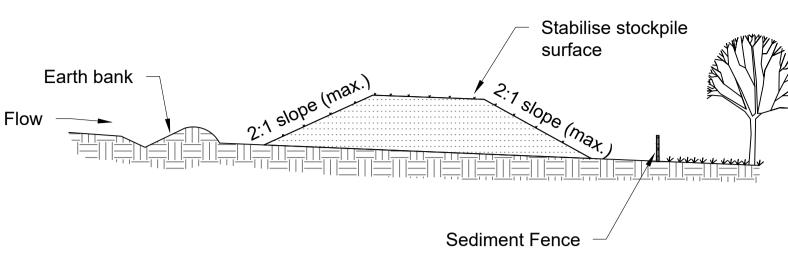


Construction Notes:

- 1. For instillation procedures for the straw bales or geo fabric Refer the NSW Managing Urban Stormwater BlueBook, Soils and Construction, Section 6.3 Std Dwg 6-7 and 6-8
- 2. In water ways, artificial sag points can be created with sand bags or earth banks.

Scale: NTS

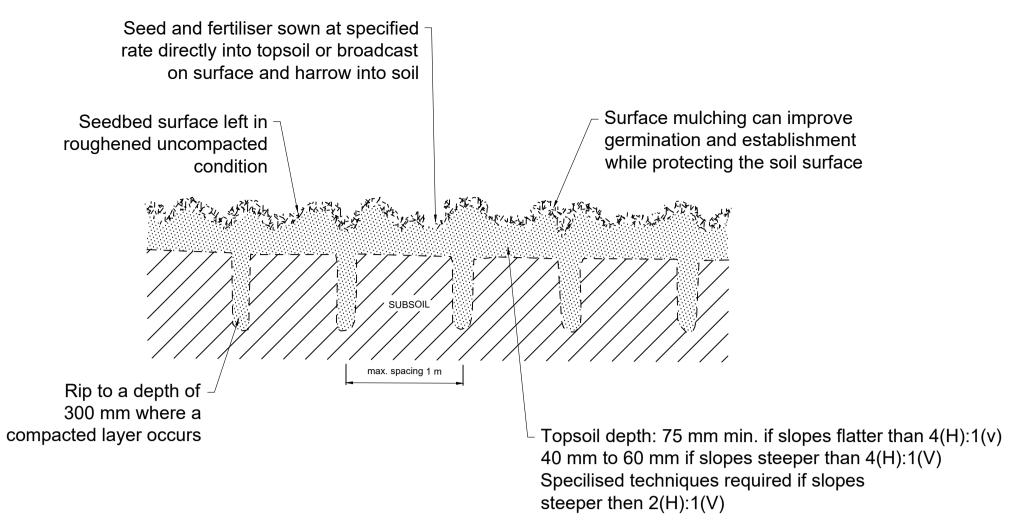
3. Do not cover the inlet with geotextile unless the design is adequate to allow for all waters to bypass it



Stockpile - SD 4-1 Scale: NTS

Construction Notes

- 1. Place stockpiles more than 2 (preferably 5) metres from existing vegetation,
- concentrated water flow, roads and hazard areas.
- 2. Construct on the contour as low, flat, elongated mounds.
- 3. Where there is sufficient area, topsoil stockpiles shall be less than 2 metres in height.
- 4. where they are to be in place for more than 10 days, stabilise following the approved ESCP or SWMP to reduce the C-factor to less than 0.10.
- 5. construct earth banks (Standard Drawing 5-5) on the upslope side to divert water around stockpiles and sediment fences (Standard Drawing 6-8) 1 to 2 metres downslope.

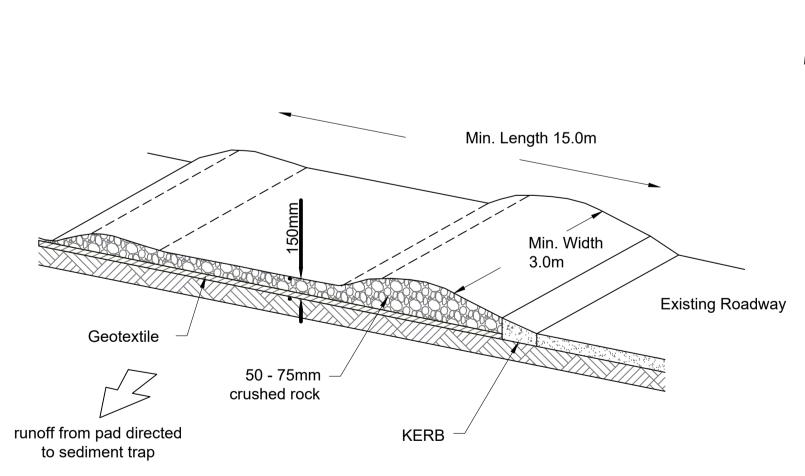


SEEDBED PREPARATION - SD 7-1

SCALE: NTS

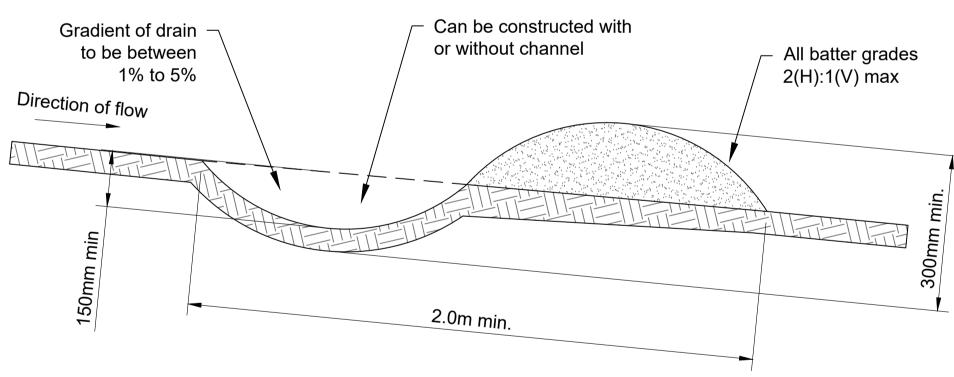
Construction Notes:

- Loosen compacted soil before sowing any seed. If necessary, rip the soil to a depth of 300 mm. Avoid rotary hoe cultivation.
- . Work the ground only as much as necessary to achieve the desired tilth and prepare a good seedbed.
- 3. Avoid cultivation in very wet ir very dry conditions.
- 4. cultivation on or close to the contour where possible, not up and down the slope.



Stabilised Site Access Detail - SD 6-14
Scale: NTS

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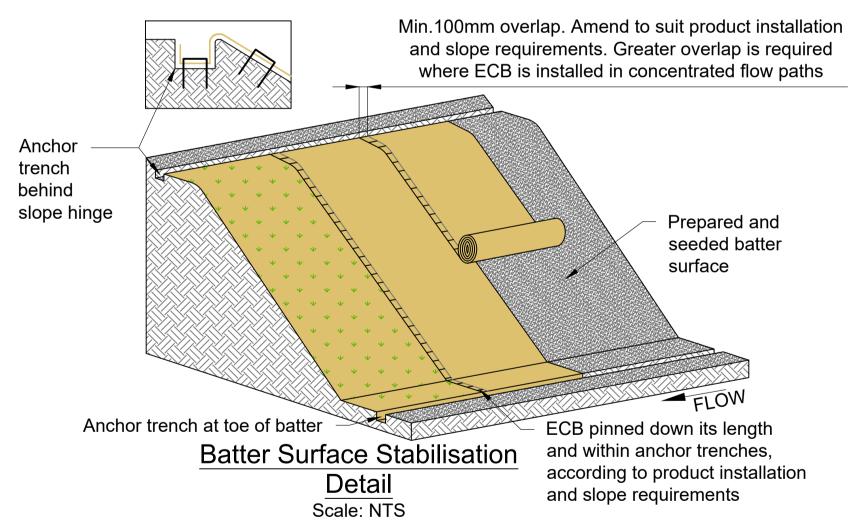
Earth Bank Detail - SD 5-5
Scale: NTS

NOTE:

Only to be used as temporary bank where maximum upslope length is 80 meters

Construction Notes:

- 1. Avoid removing trees and shrubs if possible work around them.
- 2. Ensure the structures are free of projections or other irregularities that could impede water flow
- . Build drains with circular, parabolic or trapezoidal cross sections, not V shaped
- 4. Ensure banks are properly compacted to prevent failure
- 5. Complete permanent or temporary stabilisation within 10 days of construction



NOTE:

Top soiled and seeded then lined with 350gsm jute matting and sprayed with a polymer soil stabiliser to the manufacturers specification. Polymer may require reapplication after rain events pending performance inspections until germination and suitable growth has been achieved. Construction Notes:

- 1. Avoid removing trees and shrubs if possible work around them.
- 2. Ensure 100mm min overlap of 350gsm jute matting.
- 3. Must be adequately anchored at the top of the batter, pinned / stapled and secured down the batter face at max. 300mm spacings to ensure intimate soil contact.

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| Α | 25/09/23 | FOR APPROVAL | JB | | | | | APEC Engineer IntPE (Aus) RE |
| В | 29/09/23 | REVISED SITE LEVELS AND EARTHWORKS VOLUMES | TM | | | | | Garth Dean B.E. GDSTT FIEAust CPEng N |
| С | 06/12/23 | BULK EARTHWORKS PLANS - ISSUED FOR APPROVAL | TM | | | | | 0 11 0 |
| D | 11/01/24 | FULL PLAN SET - ISSUED FOR APPROVAL | TM | | | | | |
| Е | 16/01/24 | AMENDED NOTATIONS | TM | | | | | |
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PROPOSED SPORTS COMPLEX O'CONNELL ROAD OBERON NSW 2787

SOIL & WATER MANAGEMENT NOTES

CROSSMULLER

CALARE CONSULTING ENGINEERS

____ 170 RANKIN STREET,
BATHURST, N.S.W. 2795
Tel: (02) 63323343 Fax: (02) 63318210

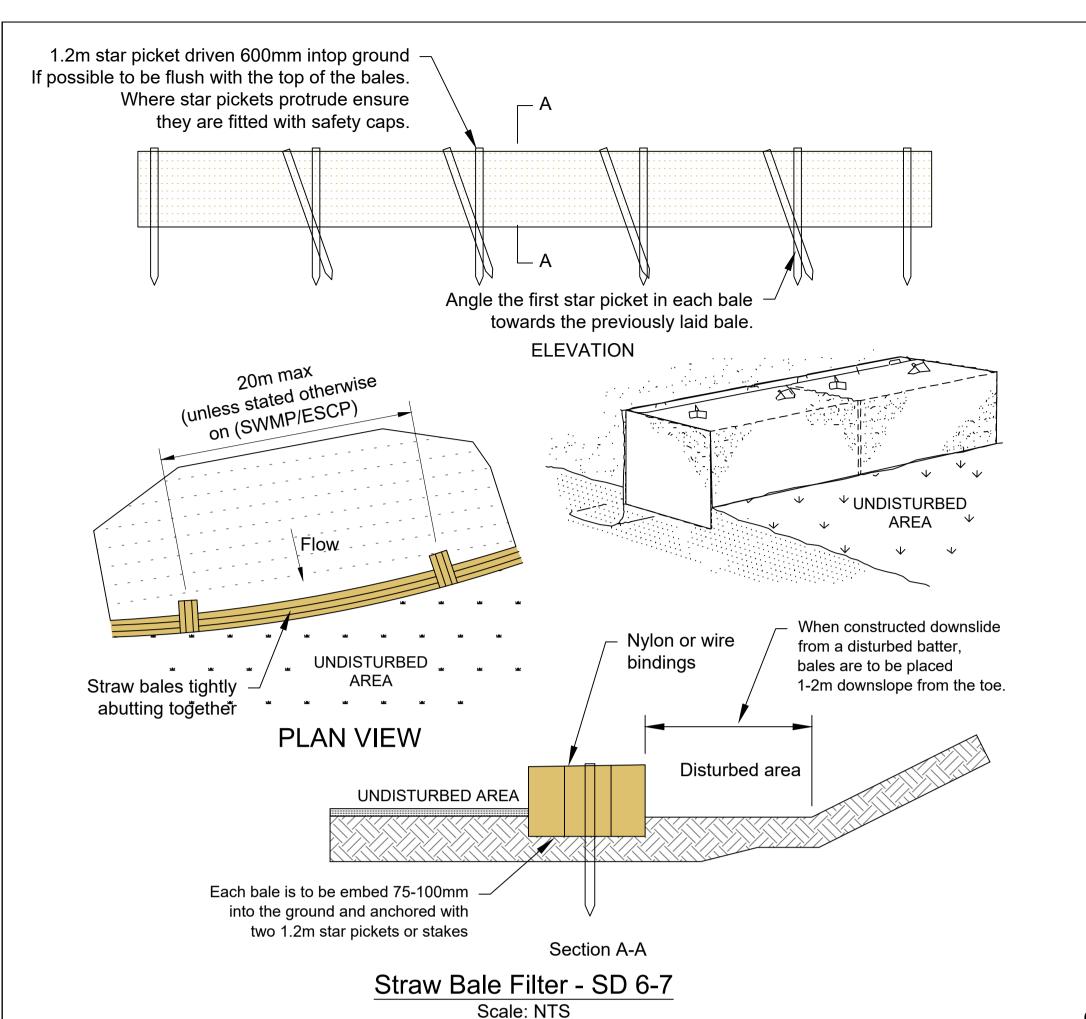
Job No.

2023.0913

DWG. No. Issue

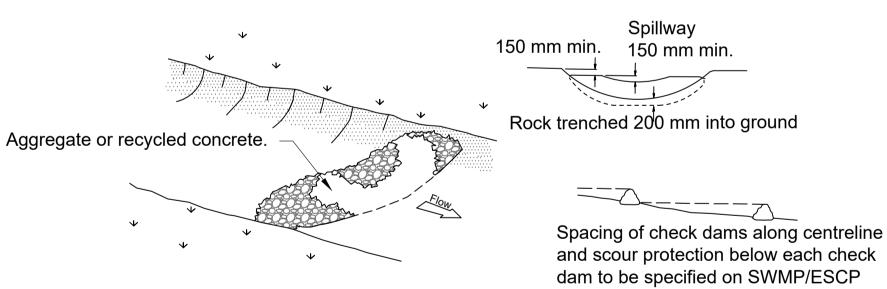
ES02 E

No. in 15



Construction Notes:

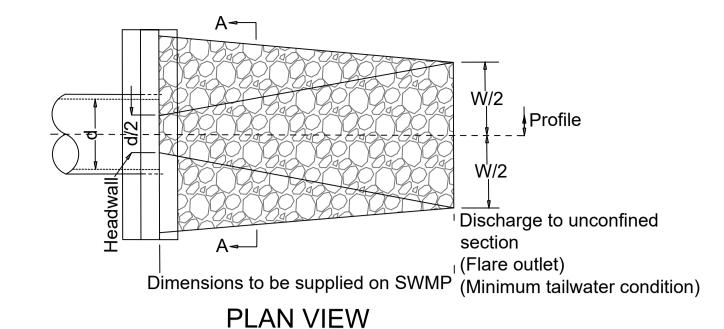
- Construct as close as possible to be parallel to site contours.
- Place bales lengthwise in a row with ends tightly abutting.
 Use straw to fill any gaps between bales.
- 3. Ensure that the maximum height of the filter is one bale.

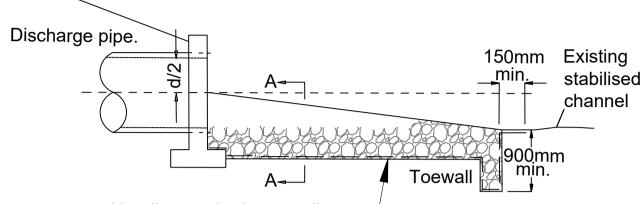


Rock Check Dam - SD 5-4 Scale: NTS

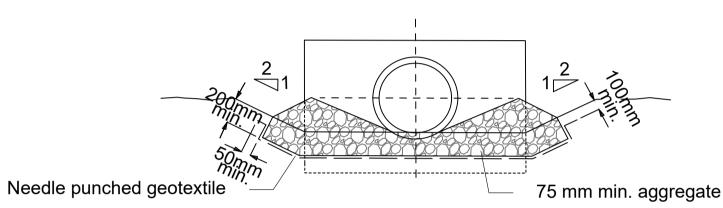
Construction Notes

- 1. Check dams can be built with various materials, including rocks, logs, sandbags and straw bales. The maintenance program should ensure their integrity is retained, especially where constructed with straw bales. In the case of bales, this might require their replacement each two to four months.
- 2. Trench the check dam 200 mm into the ground across its whole width. Where rock is used, fill the trenches to at least 100 mm above the ground surface to reduce the risk of undercutting.
- 3. Normally, their maximum height should not exceed 600 mm above the gully floor. The centre should act as a spillway, being at least 150 mm lower than the outer edges.
- 4. Space the dams so the toe of the upstream dam is level with the spillway of the next downstream







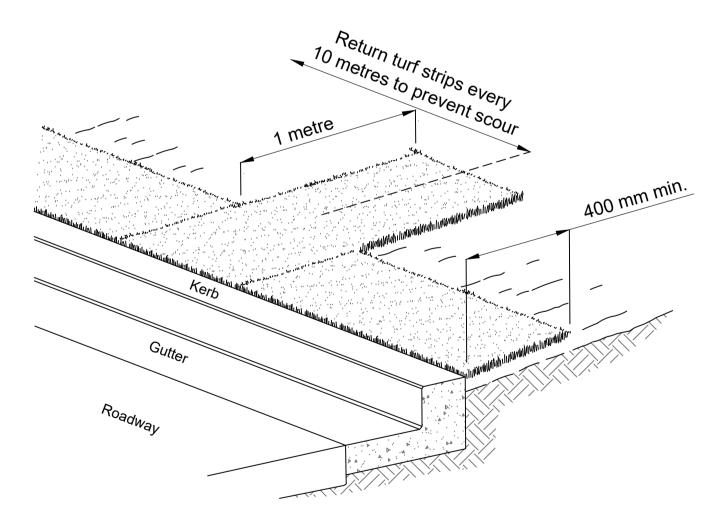


CROSS SECTION AA

Energy Dissipater - SD 5-8 Scale: NTS

Construction Notes:

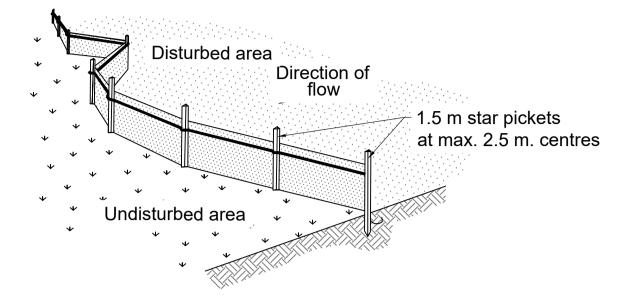
- 1. Compact the subgrade fill to the density of the surrounding undisturbed material.
- 2.Prepare a smooth, even foundation for the structure that will endure that the needle-punched geotextile does not sustain serious damage when covered with rock.
- 3.Should any minor damage to the geotextile occur, repair ir before spreading any aggregate. For repairs, patch one piece of fabric over the damage, making sure that all joints and patches overlap more than 300 mm.
- 4.Lay rock following the drawing, according to Table 5.2 of Landcom (2004) and with a minimum diameter of 75 mm.
- 5.Ensure that any concrete orriprap used for the energy dissipater or the outlet protection conforms to the grading limits specified on the SWMP.

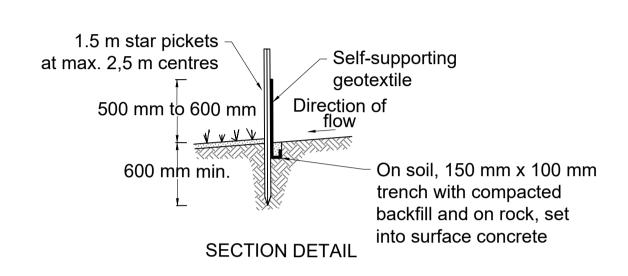


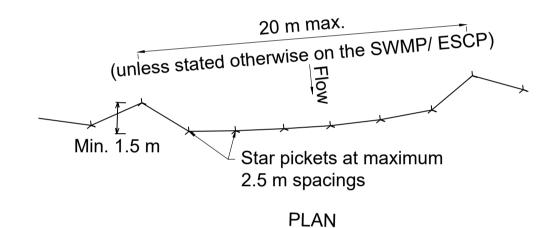
KERBSIDE TURF STRIP - SD 6-13 SCALE: NTS

Construction Notes:

- 1. Install a 400 mm minimum wide roll of turf onthe footpath next to the kerb and at the same level as the top of the curve.
- 2.Lay 1.4 metre long turf strips normal to the kerb every 10 metres.
- 3. Rehabilitate disturbed soil behind the kerb.







Sediment Fence - SD 6-8

Scale: NTS

Construction Notes:

- 1. Construct sediment fences as close as possible to being parallel to the contours of the site, but with small returns as shown in the drawing to limit the catchment area of any one section. The catchment area should be small enough to limit water flow if concentrated at one point to 50 litres per second in the design storm event, usually the 10% AEP.
- 2. Cut a 150 mm deep trench along the upslope line of the fence for the bottom of the fabric to be entrenched.
- 3. Drive 1.5 metre star pickets into the ground at 2.5 metre intervals (max) at the downslope edge of the trench. Ensure any star pickets are fitted with safety caps.
- 4. Fix self-supporting geotextile yo the upslope side of the posts ensuring it goes to the base of the trench. Fix the geotextle with wire ties or as recommended by the manufacturer. only use geotextile specifically produced for sediment fencing. the use of shade cloth for this purpose is not satisfactory.
- 5. Join sections of fabric at a support post with a 150 mm overlap.
- 6. Backfill the trench over the base of the fabric and compact it throrughly over the geotextile.

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PROPOSED SPORTS COMPLEX
O'CONNELL ROAD
OBERON NSW 2787

CROSSMULLER

SOIL & WATER MANAGEMENT NOTES

CALARE CHIMI CONSULTING ENGINEERS

170 RANKIN STREET,

BATHURST, N.S.W. 2795

Tel: (02) 63323343 Fax: (02) 63318210

Notes: Sediment Control

- I. Erosion and sedimentation control shall be in accordance with NSW landcom guidelines " managing urban stormwater soils & construction" 4th edition. Minimise disturbance of existing vegetation during construction. Erosion & sediment controls to be in place prior to any construction work commencing.
- 2. Construction is to be programmed to provide installation of perimeter landscaping/surface treatment as early as practical.
- 3. At the prestart meeting the contractors works program is to be reviewed.

 Alterations to the program may be required to ensure satisfactory erosion and sediment control.
- 4. A photographic record of sediment and erosion control devices and the immediate downstream stormwater system is to be carried out on a fortnightly cycle and after each major storm event. Carry out corrective and preventative action as required.
- 5. Public and workplace safety issues must be considered and monitored for each device to the satisfaction of the superintendent.
- 6. Woven fabrics are to be used for sediment fence filter fabric.
- Sediment management devices shall be installed prior to commencement of construction activities and maintained at a suitable level/condition throughout construction. Sediment fences are to be cleaned out when capacity is reduced by 30%. Drainage structure protection is to be cleaned following each significant runoff producing storm.
- 8. The contractor shall provide temporary drainage control to divert flow from undisturbed areas around disturbed areas and direct flow from disturbed areas towards control devices.
- The contractor shall be responsible for the inspection and maintenance of sediment and erosion control devices. All devices are to be inspected at least weekly and after significant runoff producing storms.
- 10. If erosion and sediment control devices have been found to be deficient or failed in service due to unforeseen circumstances corrective action is to be undertaken by the contractor immediately which may include; amendments/additions to the original erosion control plans. Such additions or amendments are to be approved by the superintendent.
- 11. Straw bales used in sediment devices are to be replaced after a maximum service period of 6 weeks or as required.
- 12. Sediment management devices are to be maintained by the contractor as noted and detailed until approval has been granted by the engineer for their removal. The contractor is to remove and dispose of these devices off site.
- 13. All temporary access roads and hardstand areas are to be trimmed and maintained in a serviceable condition for the duration of the contract.
- 14. All temporary access roads and hardstand areas are to be reinstated to the satisfaction of the superintendent at the end of the contract.

Dust Management

- 1. Ground disturbance is to be minimised and all site vehicle movements are to be maintained with the designated haulage tracks and or roads.
- 2. All site traffic speeds are to be kept to a minimum. Maximum speed 10 kph.
- 3. Water tankers are to be kept onsite for the duration of works and until seeded areas are considered stabilised or when ground cover achieves 70% plant density to at least 100mm in height.
- 4. The contractor will ensure that haul roads and all denuded areas are watered as required and a trackifier such as curosol may be required.
- 5. In the event that dust becomes a nuisance council may instruct the contractor to cease all work until a satisfactory control has been reached.

Revegetation Management

- 1. All flat bottom drains to be top soiled and seeded then covered with 350gsm jute matting or similar and sprayed with polymer soil stabiliser at the manufacturers recommended rate.
- 2. All batters & reinstatement works adjacent to new construction works shall be carried out as soon as possible after completion.
- 3. All disturbed areas & batters shall be turfed or grassed after reinstatement and achieve 70% cover after 10 working days. Areas not worked for 20 days must achieve 50% cover.
- 4. Replace topsoil on all disturbed areas to a depth of at least 75mm depth on slopes less than 4h:1v and 40mm to 60mm on lands where slopes exceed 4h to 1v.
- 5. Sow or hydromulch disturbed areas with approved seed/fertiliser mixture.

Largest Drain Calculations Adopted For All Clean/Dirty Water Drains

Clean/Dirty water drains calculated as a swale to contain a 10% AEP event:

Maximum catchment area 4.84ha
Calculated runnoff 0.61m³/s
Minimum longitudinal drain slope -1.0%
Downhill side slope 1:4

Uphill slope 1:4
Bottom of drain width 600mm (Bund)
Flow depth 290mm

Flow depth 290mr
Calculated depth plus 1/3 freeboard 0.4m

- All drain/bund mounds to be compacted to minimum 95% standard compaction, unless topsoil bund, then hand roller only. Drains must be maintained and kept clear of sediment build up.
- All diversion drains/bunds are to be top soiled and seeded then lined with 350gsm jute matting and sprayed with a polymer soil stabiliser to the manufacturers specification. Polymer may require reapplication after rain events pending performance inspections until germination and suitable growth has been achieved.
- All interallotment and open drain surface inlet pits to have geotextile filters similar to SD 6-12 or geotextile pit nappies until area stabilised.

Document inspection of all controls weekly and after each rain event.

Erosion & Sedimentation Controls

Design storm event - 10% AEP

As per table 4.3 of the Landcom Managing Urban Stormwater: Soils and Construction, Zone 7 Soil Loss Class 1 is not restrictive for construction timing

1. Erosion Hazard and Sediment Basins

Site Name: Oberon Sports Complex

Site Location: Oberon

Precinct/Stage:

Other Details:

| Site area | Sub- | catchm | ent or | Name | Notes | |
|-------------------------------|-------|--------|--------|------|-------|-------|
| Sile area | | | | | | Notes |
| Total catchment area (ha) | 12.94 | | | | | |
| Disturbed catchment area (ha) | 8.5 | | | | | |

Soil analysis (enter sediment type if known, or laboratory particle size data)

| Sediment Type (C, F or D) ii known. | U | | | From Appendix C (ii known) |
|---------------------------------------|---|--|--|---------------------------------------|
| % sand (fraction 0.02 to 2.00 mm) | | | | Enter the percentage of each soil |
| % silt (fraction 0.002 to 0.02 mm) | | | | fraction. E.g. enter 10 for 10% |
| % clay (fraction finer than 0.002 mm) | | | | naction. E.g. enter 10 to 1070 |
| Dispersion percentage | | | | E.g. enter 10 for dispersion of 10% |
| % of whole soil dispersible | | | | See Section 6.3.3(e). Auto-calculated |
| Soil Texture Group | D | | | Automatic calculation from above |

Rainfall data

| Design rainfall depth (no of days) | 5 | | | See Section 6.3.4 and, particularly, |
|---|------|--|--|--|
| Design rainfall depth (percentile) | 80 | | | Table 6.3 on pages 6-24 and 6-25. |
| x-day, y-percentile rainfall event (mm) | 22.5 | | | Table 0.0 of pages of 24 and 6 20. |
| Rainfall R-factor (if known) | | | | Only need to enter one or the other here |
| IFD: 2-year, 6-hour storm (if known) | 6.16 | | | orny flood to office of the other flore |

RUSLE Factors

| NOSEE Factors | | | | | | | |
|--------------------------------------|-------|-----|-----|-----|-----|-----|---------------------------------------|
| Rainfall erosivity (R-factor) | 1060 | | | | | | Auto-filled from above |
| Soil erodibility (K -factor) | 0.035 | | | | | | |
| Slope length (m) | 17 | | | | | | |
| Slope gradient (%) | 25 | | | | | | RUSLE LS factor calculated for a high |
| Length/gradient (LS -factor) | 2.84 | | | | | | rill/interrill ratio. |
| Erosion control practice (P -factor) | 1.3 | 1.3 | 1.3 | 1.3 | 1.3 | 1.3 | |
| Ground cover (C-factor) | 1 | 1 | 1 | 1 | 1 | 1 | |

Sediment Basin Design Criteria (for Type D/F basins only. Leave blank for Type C basins)

| Storage (soil) zone design (no of months) | 2 | | | Minimum is generally 2 months |
|---|-----|--|--|--------------------------------------|
| Cv (Volumetric runoff coefficient) | 0.5 | | | See Table F2, page F-4 in Appendix F |

Calculations and Type D/F Sediment Basin Volumes

| Soil loss (t/ha/yr) | 137 | | | |
|--|------|--|--|--|
| Soil Loss Class | 1 | | | See Table 4.2, page 4-13 |
| Soil loss (m³/ha/yr) | 105 | | | Conversion to cubic metres |
| Sediment basin storage (soil) volume (m ³) | 149 | | | See Sections 6.3.4(i) for calculations |
| Sediment basin settling (water) volume (m ³) | 1456 | | | See Sections 6.3.4(i) for calculations |
| Sediment basin total volume (m ³) | 1605 | | | |

Soil Loss is less than 200 tonne (~150m³) therefore a sediment basin is NOT required.

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| Amend | Date | Description | Ву | Amend | Date | Description | Ву | (Vic/NT) |
|-------|----------|---|----|-------|------|-------------|----|---|
| Α | 25/09/23 | FOR APPROVAL | JB | | | | | APEC Engineer IntPE (Aus) RB |
| В | 29/09/23 | REVISED SITE LEVELS AND EARTHWORKS VOLUMES | TM | | | | | Garth Dean B.E. GDSTT FIEAust CPEng NE |
| С | 06/12/23 | BULK EARTHWORKS PLANS - ISSUED FOR APPROVAL | TM | | | | | 0 4 5 |
| D | 11/01/24 | FULL PLAN SET - ISSUED FOR APPROVAL | TM | | | | | |
| E | 16/01/24 | AMENDED NOTATIONS | TM | | | | | |
| | | | | | | | | |
| | | | | | | | | Approved for Construction: |

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Designed:

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Scale (A1):

Date:

TM
GBL

AS SHOWN
25/09/23

TM
FOR APPROVAL

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PROPOSED SPORTS COMPLEX
O'CONNELL ROAD
OBERON NSW 2787

SOIL & WATER MANAGEMENT NOTES

CROSSMULLER

CALARETH VILLE CONSULTING ENGINEERS

Tel: (02) 63323343 Fax: (02) 63318210

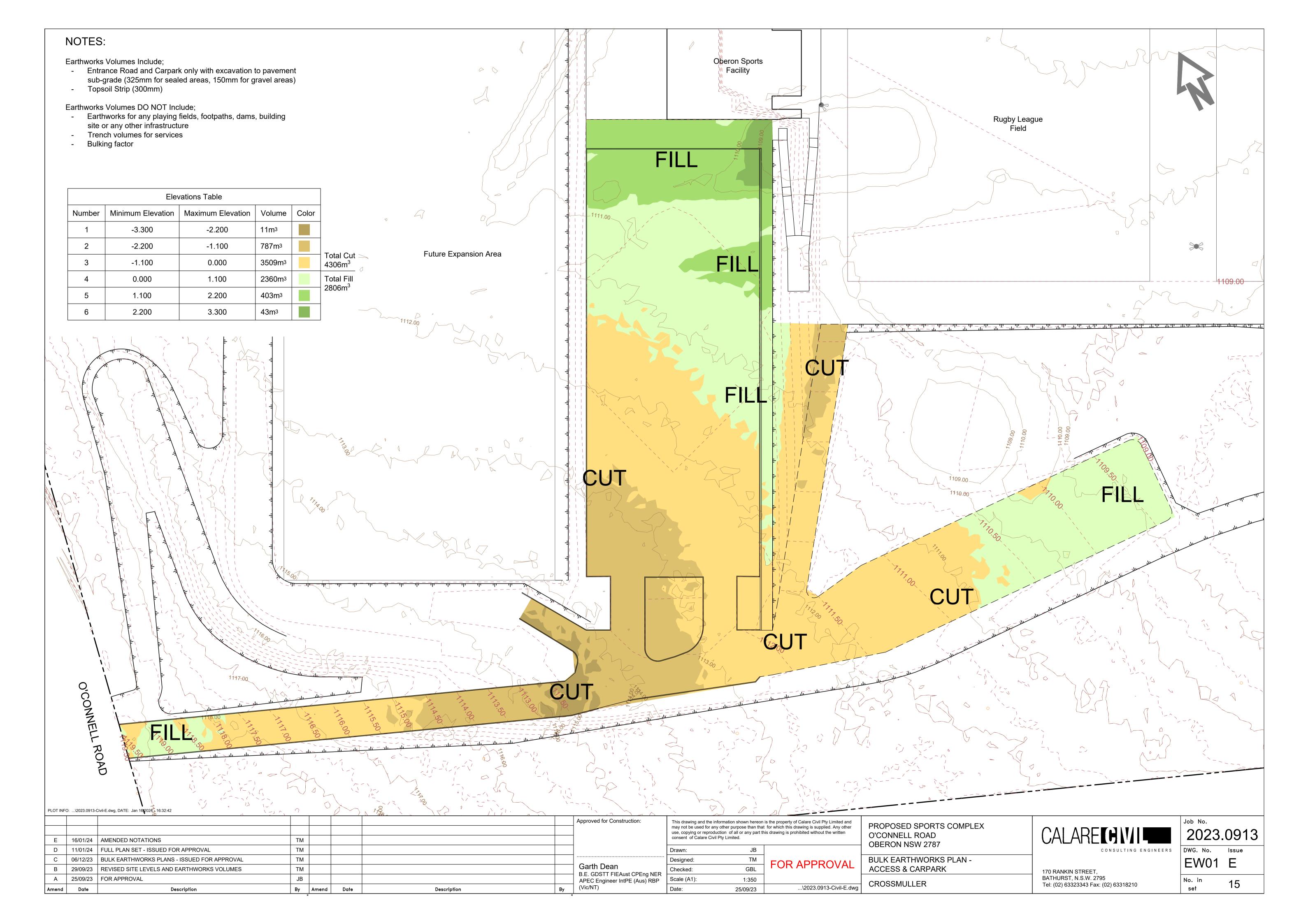
170 RANKIN STREET, BATHURST, N.S.W. 2795 Job No.

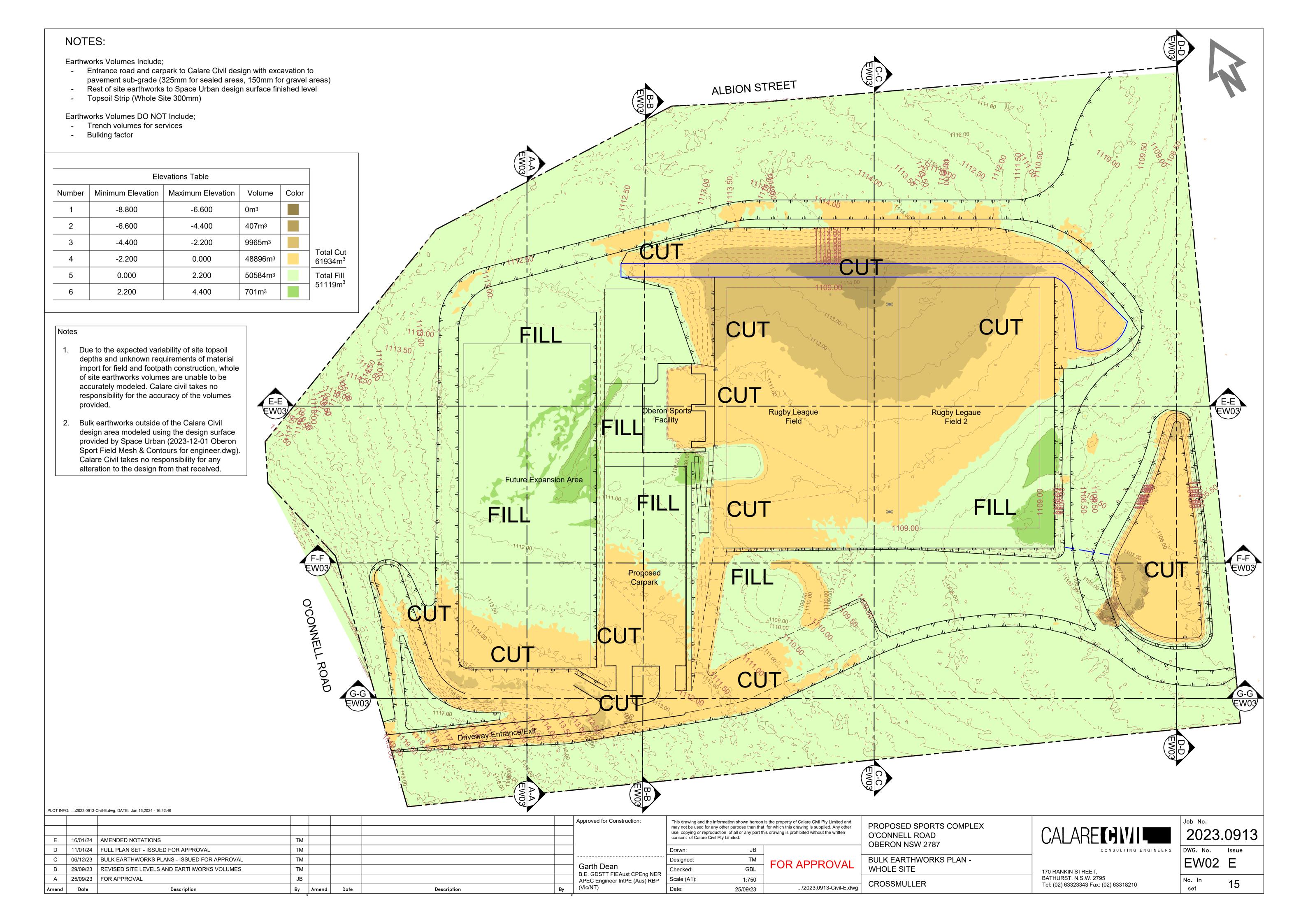
2023.0913

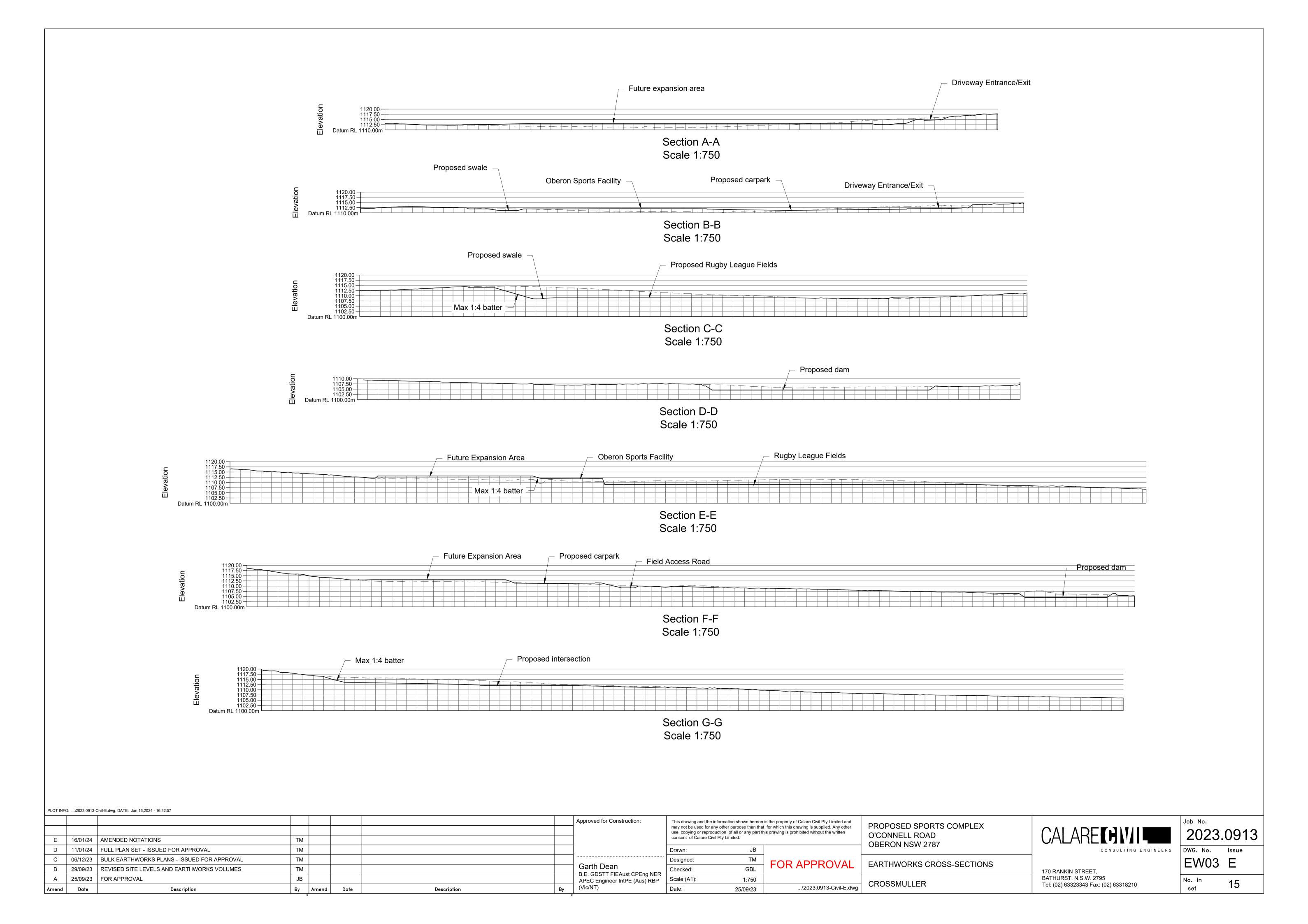
DWG. No. Issue

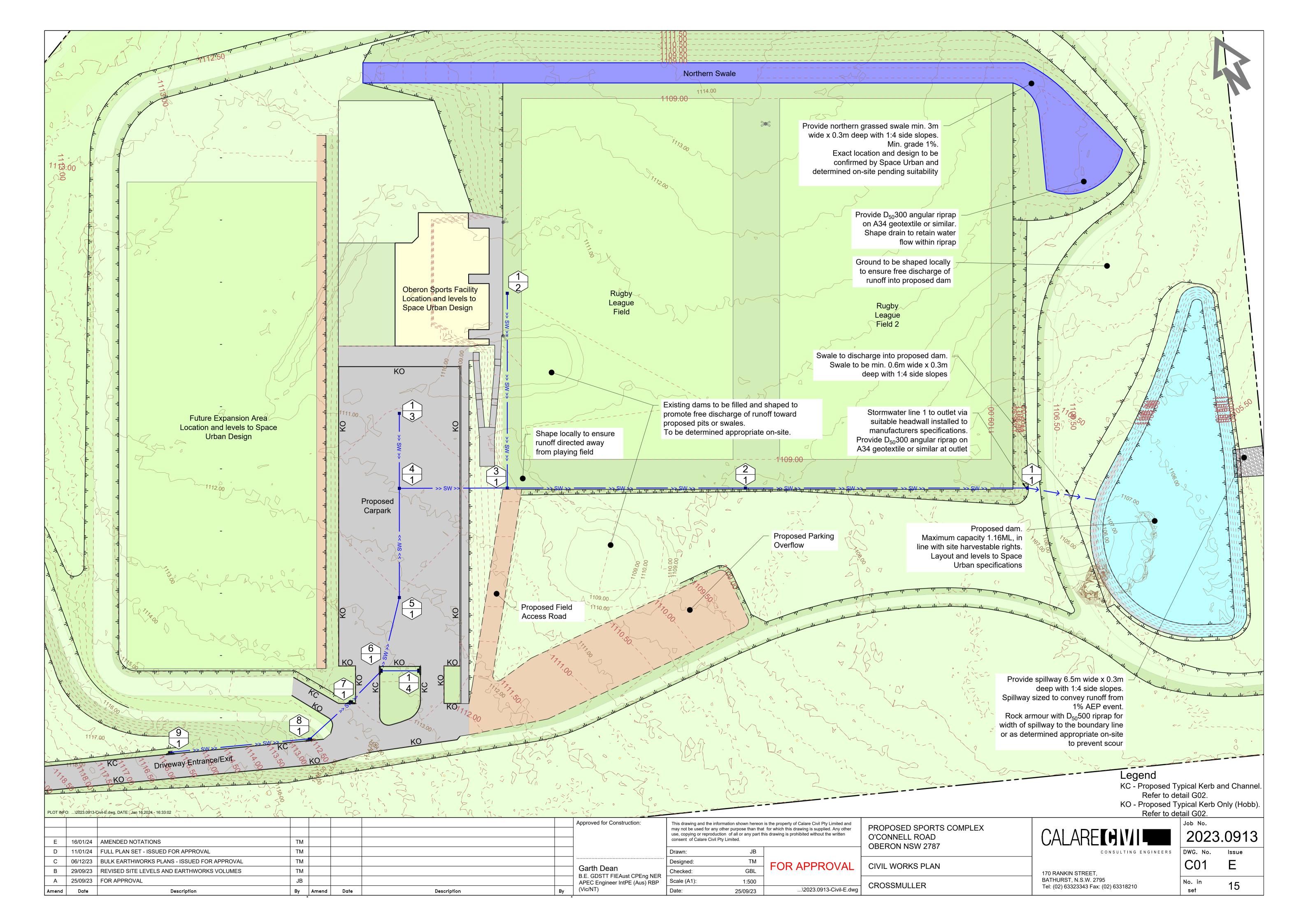
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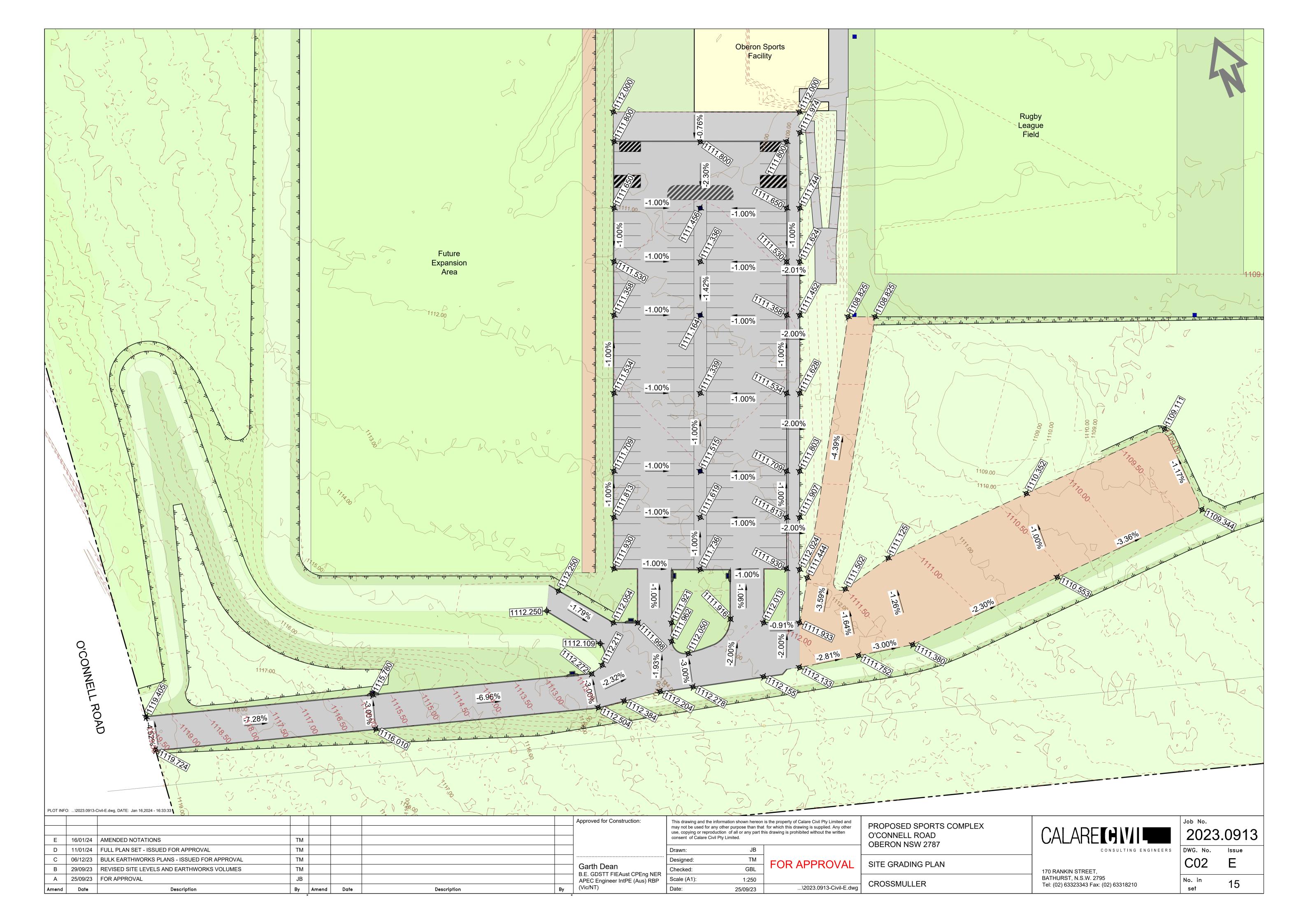
No. in
2023.0913











Overland Flow Path Calculation - Northern Swale

| | Mannings Channel Calculations | | | | | | | | | | | | |
|--------------|-------------------------------|------------------|------------|-----------|-----------|-----------|------------|-----------|--|--|--|--|--|
| | | | | | | | | | | | | | |
| => | Q | 1.92 | (m3/sec) | | | | | | | | | | |
| Mannings | | | | | | | | | | | | | |
| | $V=(1/n)R^{2/3}$ | S ^{1/2} | R=A/P | | Q=AV | | | | | | | | |
| | n = | 0.025 | Grassed | | | | | | | | | | |
| Where: | H = depth | of channel | P = wetted | parameter | A = Secti | onal Area | | | | | | | |
| | S = slope | of channel | | | | | | | | | | | |
| | | | | | | | | | | | | | |
| Width of ba | ase | | 3 | m | | | Top Width | 5.4 | | | | | |
| Batter slope | e (left) | 1: | 4 | | | | | | | | | | |
| Batter slop | e (Right) | 1: | 4 | | | | | | | | | | |
| | | | | | | | | | | | | | |
| | CATCHMENT 1 - OVERFLOW 1 | | | | | | | | | | | | |
| | | | | | | | to 2% | | | | | | |
| Н | Р | Α | S | R | V | Q | tollerance | VD | | | | | |
| 0.3 | 5.4738634 | 1.26 | 0.01 | 0.2301848 | 1.5023728 | 1.8929897 | OK | 0.4507118 | | | | | |
| | 3 | 0 | 0.01 | 0 | 0 | 0 | Low | 0 | | | | | |
| | 3 | 0 | 0.01 | 0 | 0 | 0 | Low | 0 | | | | | |

Overflow Calculation - Dam Spillway

| | | | | odiation | op | | | |
|-------------|------------------|------------------|------------|-----------|-----------|-----------|------------|-----------|
| | | Man | nings C | hannel | Calcula | tions | | |
| => | Q | 3.57 | (m3/sec) | | | | | |
| Mannings | | | | | | | | |
| | $V=(1/n)R^{2/3}$ | S ^{1/2} | R=A/P | | Q=AV | | | |
| | n = | 0.025 | Grassed | | | | | |
| Where: | H = depth | of channel | P = wetted | parameter | A = Secti | onal Area | | |
| | S = slope | of channel | | | | | | |
| | | | | | | | | |
| Width of ba | ase | | 6.5 | m | | | | |
| Batter slop | e (left) | 1: | 4 | | | | | |
| Batter slop | e (Right) | 1: | 4 | | | | | |
| | | | | | | | | |
| | | CATO | HMENT 1 | - OVERFI | _OW 1 | | | |
| | | | | | | | to 2% | |
| Н | Р | Α | S | R | V | Q | tollerance | VD |
| 0.28 | 8.8089392 | 2.1336 | 0.01 | 0.2422085 | 1.5542453 | 3.3161378 | Low | 0.4351887 |
| 0.3 | 8.9738634 | 2.31 | 0.01 | 0.2574142 | 1.6186327 | 3.7390416 | High | 0.4855898 |
| 0.32 | 9.1387876 | 2.4896 | 0.01 | 0.2724213 | 1.6809468 | 4.1848851 | High | 0.537903 |

20% AEP Results

| SUB-CATO | CHMENT DE | TAILS | | | | | | | | |
|----------|-------------|----------|-----------|----------|-------|-------|----|------------|------------|-------------|
| Name | ame Max EIA | | Remaining | EIA | RIA | PA | | Due to Sto | orm | |
| | Flow Q | Max Q | Max Q | Тс | Тс | Тс | | | | |
| | (cu.m/s) | (cu.m/s) | (cu.m/s) | (cu.m/s) | (min) | (min) | | (min) | | |
| Ex-Cat | 1.862 | 0 | 1.862 21 | | 2 | | 21 | 20% AEP, | 20 min bur | st, Storm 9 |
| Dev-Cat | 1.863 | 0.28 | 1.583 | 1.583 21 | | | 21 | 20% AEP, | 20 min bur | st, Storm 5 |

1% AEP Results

| SUB-CATO | CHMENT DE | TAILS | | | | | | | | | |
|----------|------------|----------|-----------|----------|-------|-------|--------------|-------------|-------------|--|--|
| Name | ne Max EIA | | Remaining | EIA | RIA | PA | Due to Storm | | | | |
| | Flow Q | Max Q | Max Q | Тс | Тс | Тс | | | | | |
| | (cu.m/s) | (cu.m/s) | (cu.m/s) | (cu.m/s) | (min) | (min) | (min) | | | | |
| Ex-Cat | 3.567 | 0 | 3.567 | 21 | 2 | 21 | 1% AEP, 2 | 0 min burst | st, Storm 6 | | |
| Dev-Cat | 3.568 | 0.536 | 3.032 | 21 | 2 | 21 | 1% AEP, 2 | 0 min burst | t, Storm 4 | | |

• An IL/CL model has been run on Watercom Drains, to determine the increase in site runoff in the post development scenario. This demonstrated a negligible increase in runoff in the 20% AEP and 1% AEP rainfall events, and therefore no stormwater detention is proposed for the development

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| Amend | Date | Description | Ву | Amend | Date | Description | Ву | (Vic/N |
| Α | 25/09/23 | FOR APPROVAL | JB | | | | | APEC |
| В | 29/09/23 | REVISED SITE LEVELS AND EARTHWORKS VOLUMES | TM | | | | | Gartl |
| С | 06/12/23 | BULK EARTHWORKS PLANS - ISSUED FOR APPROVAL | TM | | | | |] |
| D | 11/01/24 | FULL PLAN SET - ISSUED FOR APPROVAL | TM | | | | | |
| Е | 16/01/24 | AMENDED NOTATIONS | TM | | | | | |
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| | | | | | | | | _ , .bb.o. |

Approved for Construction:

Designed: earth Dean
E. GDSTT FIEAust CPEng NER
PEC Engineer IntPE (Aus) RBP Checked: Scale (A1): Date:

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PROPOSED SPORTS COMPLEX O'CONNELL ROAD **OBERON NSW 2787**

STORMWATER CALCULATIONS

CROSSMULLER

...\2023.0913-Civil-E.dwg

CONSULTING ENGINEERS

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